Membrane Roof Covering Parts of C.U. Sports Center Swimming Pool

Master - Thesis

A Thesis submitted in partial fulfillment of the requirements for the degree of

Master Membrane Structures

submitted to

Anhalt University of Applied Sciences

Faculty of Architecture Facility Management and Geo Information

by

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Statement

I hereby declare that the work presented in this Master Thesis, entitled: "*Membrane Roof Covering Parts of Chulalongkorn University (C.U.) Sports Center Swimming Pool*" is entirely my own and that I did not use any sources or auxiliary means other than those referenced.

Bangkok, Thailand. 11 September 2011

CONTENT

1. PROJECT INTRODUCTION

PROJECT INTRODUCTION

Background

Chulalongkorn University, Thailand's first institution of higher learning, located in Thailand's capital city Bangkok, was officially came into being in March, 1917 by King Vajiravudh, Rama VI. The university is named after his father King Chulalongkorn, Rama V. At first, the university was under the supervision of University Affairs Department, Ministry of Education. Phraya Anukijwithoon was the first principal.

When it was first found, the university had only 380 students taking classes in four faculties. After the decades had passed, Chulalongkorn University was continuing developed, it extends over 456 acres on land granted by the royal family and composed of 41 faculties, departments, colleges, and research offices. The student body consists of nearly 21,000 undergraduate students, 9,740 master degree students, and 1,737 doctorial students (2009). Furthermore, Chulalongkorn university was ranked 138th in the World University Ratings by Times Higher Education.

To meet the needs of the university community, faculty, personnel, and students, the university has built a number of facilities and established various services such as healthcare, transportation, book center, and sport center.

Figure 1.1 Map of Thailand Figure 1.2 Chulalongkorn University

Figure 1.3 Swimming pool in Sport Center

Figure 1.4 Arial view of the swimming pool

Site and Existing Conditions

Chulalongkorn University Sports Center is a part of Chulalongkorn university and its campus program offerings. The center and its subsidiary facilities include the following: outdoor stadium, 2 indoor stadiums, tennis courts, football field, 2 swimming pools, fitness room, golf putting practice, beach volleyball court, sports complex, and outdoor fitness area.

This project is a part of "Sports Center Refurbishment Project" which was held on November 2010. The aim of this project is to design a new roof cover for the outdoor swimming pool deck in order to increase their functionality and to extend their use for other activities.

Figure 1.5 Map of Chulalongkorn University

Figure 1.6 Layout of the swimming pool

 $V(\mathbf{r}, \mathbf{r})$

Figure 1.7 Existing condition of the swimming pool (on North wing)

Figure 1.8 Existing condition of the swimming pool (on West wing)

The area needs to be covered is the deck in the north and west side of swimming pool. Both of them are confined by chain link steel fences which have a height of 2.00 m. The design must provide the membrane roof cover the those both sides of swimming pool deck (area approx. 150 sq.m.) without any intermediate columns. The constraint that the designer faced was selecting the materials and method of construction to suit the university's budget and could be done in short time.

Design Proposal

The conceptual design based on the concept of simplicity and functionality. Three main architectural elements: fabric membrane, cable, and steel frame were compromised with the structural system, making it a more interesting and lightweight structure.

With very long and narrow L-shape area to design, the roof, 21.5 m. by 4 m. in the north and 30 m. by 3.8 m. in the west, is comprised of 14 steel rigid frames which connected together by steel beams. Each frame has a cantilevering end, which has a length of around 4 meters. Fabric membranes were spanned over these steel structures and tensioned after assembly.

Figure 1.10 3D Model Design

Figure 1.9 Sketches of Design Concept

2. STRUCTURAL DESIGN

Form finding

The "form finding" is the starting point in the design process to find a shape of the tension membrane which approximates the state of equilibrium.

This process consisted of experimental work on small-scale computational model which was analysed by Forten 4000. The computational model represent a set of numerical and graphical data describing structure's shape, stresses, and deformations under various load conditions.

Figure 2.1 Computational Model analysed by Forten 4000

Structural Concept

The structural performance of this project based on the design concept, Simplicity and Functionality. The roof construction consists of a series of the steel load-bearing structure – rigid frame columns, which are arranged parallel running to the length of the pool (Figure 1.10). Each frame has cantilevering end with a span of 4 m. and its base is fixed to the swimming pool deck.

The roof consists of a single layer of PVC-coated polyester membrane. This translucent fabrics covering are stretched over the primary structure. In the north and west wing, the membrane were tensioned to the edges and fixed by the stainless-steel clamping strips to the steel sections whereas in the middle of membrane in each module was tied down to the column by Λ-shaped cables as a low point which provides double curvature and drainage. These cables fix the end points to the adjacent columns (figure 2.2). The steel columns were fixed at their bases in order to stabilize the overall structure. The loads from membrane are transmitted to the steel structures and from there to the ground. Codes, Loads, and structure analysis will be provided in the next section.

Figure 2.2 Structural Concept in an individual module

Load Conditions

The codes and recommendation used for analysis in this project are mentioned as following:

1. Dead Load

Dead loads include the weight of all material used in construction. These loads can be estimated by the quantity of each material and then multiplying it with unit weight. The self weight of material used in this project are listed in table 2.1

Table 2.1 Unit Weight of Materials

Construction

** See Appendix IV*

Figure 2.3 Self weight act on structure

The level of prestress in membrane depends on the type of fabric, and external loading. The assume value for the prestress in the fabric is recommended by European Design Guide for Tensile Structure, which indicate that, for PVC coated polyester membrane structure, the prestress should not be less than 1.3 % nor greater than 6 % of the average tensile strip capacity of the material in warp and weft direction.

According to Technical characteristic of membrane (see Appendix III), for PVC type I, Precontraint 702, the approximate value prestress are given as:

 V_0 , min = 0.65 kN⋅m-¹

 V_0 , max = 3.00 kN⋅m-1

2. Live Load

Live loads are movable loads produced in a structure by workers or mobile equipments. Actual values of live loads are difficult to predict. In this case, according to General Building Codes of Thailand*, the minimum design live load should be provided is 30 kg⋅m-²

** Minimum Design Loads for Building and other Structures, ANSI/ASCE 7-95, American Society of Civil Engineers. (See Appendix I)*

Figure 2.4 Live load occurs on structure

3. Wind Load

Similar to other membrane architecture, although it is located in the Bangkok, where has less impact from wind condition than other regions, the wind loading is still an important design loadcase, especially the lift force, and need to be taken into account.

Wind load acts on the structure, especially in the form of uplift, can be determined according to DPT Standard 1311-50 and EIT Standard 1018-46 Codes of Thailand.

There are three different approaches for determining wind load act on structure (see appendix II). Since the height of structure in this project is not greater than 80 m. Therefore, the procedure used for determining wind load is "Simple procedure", based on the DPT Standard 1311-50.

The specified external pressure on surface can be expressed as:

$$
P = (I_w)(q)(Ce)(Cg)(Cp)
$$

- where $P =$ The specified external pressure or a suction directed away from the surface (kg⋅m-2)
	- Iw= Importance factor for wind load, as shown in Table 1
	- q = The reference velocity pressure (kg⋅m⁻²)
	- Ce = The exposure factor
	- $Cg =$ The gust effect factor
	- Cp = The external pressure coefficient

3.1 Important Factor (Iw)

According to the occupancy used (see appendix II) of the project, the structure can be categorized as normal building, ULS. Hence, $\boldsymbol{\mathsf{I}}_{_{\mathsf{W}}}$ = 1.0 is used. (Table 2.2)

Source: Virote Boonyapinyo, 2010.

3.2 Reference Velocity Pressure (q)

The reference wind velocity is determined from reference wind speed, V by the following equation:

$$
q = \frac{1}{2} \left(\frac{\rho}{g} \right) \overline{V}^2
$$

whereq = Reference wind velocity pressure (kg⋅m-2)

- Air density (1.25 kg⋅m-3)
- $g =$ Acceleration due to gravity (9.81 m⋅s⁻²)
- $V =$ Design wind speed (m⋅s⁻²)

(see detail in Appendix II)

The project is located in Bangkok which located in zone I, Central region. Hence, V_{50} = 25 and T_F = 1.0 are used.

Therefore, q =
$$
\frac{1}{2} \left(\frac{1.25}{9.81} \right) (25)^2
$$

= 39.82 kg·m⁻²

3.3 Exposure Factor (Ce)

The exposure factor (Ce) used in Simple procedure are given in Table 2.3

Table 2.3 Exposure Factors (Ce)

Source: Virote Boonyapinyo, 2010.

Since the maximum height of the structure is 4.50 m. Therefore, the exposure factor selected from the table 2.3 is 0.9.

3.4 Gust Effect Factor (Cg)

The gust effect factor (Cg) is defined as the ratio of the maximum effect of the loading to the mean effect of the loading. In this case, the gust effect factor used for simple procedure is 2.0 (for the building as a whole and main structural members; see appendix II).

3.5 Pressure Coefficient (Cp)

According to the roof geometry, the angle of roof which incline to the horizontal plane is approximately 5 degrees and the roof mean height –to- the roof cantilever is equal to 1.0. as shown in figure 2.6. Therefore, the pressure coefficient value can be taken from the table 2.4

Figure 2.6 Wind direction against to the structure (a) Plan (b) Elevation

Table 2.4 Exposure Factors for windward roof, θ < 10°

From the table 2.4, the pressure coefficient values suit for the roof are 1.3 and 0.7 (suction) which act on the structure by the distance of 1.7 m and 2.25 m. respectively (Figure 2.7).

From $P = (I_w)(q)(Ce)(Cg)(Cp)$

 $P_{(Cp = -1.3)}$ = (1)(39.82)(0.9)(2)(-1.3) = -93.17 kg⋅m⁻² $P_{(Cp = -07)}$ = (1)(39.82)(0.9)(2)(-0.7) = -50.17 kg⋅m⁻²

According to the site orientation. Two sides of rectangular shape are enclosed by the surround buildings but the others face to the open spaces, Thus the wind directions which need to take into account remain in 2 sides as shown in the picture:

Figure 2.9 Building orientation and Wind direction

Load Combination

To carry out the structural design, it is necessary to quantify the effects of the loads which will be exerted on each element of the structure. The design loads to be considered in this project are as follows:

- 1) Prestress (V_0) 2) Dead load (DL) 3) Live load (LL)
- 4) Wind load (WL)

In realistic situation, these load types may act on a structure. Fortunately, Thailand is located in a tropical region. Thus, the calculation of snow accumulation is not required, only wind conditions on structure must take into account. Therefore, according to DIN EN 1990 the load factors and load cases could be managed as in table 2.5

Table 2.5 Load combination and load cases

Source: Dr.-Ing. Karsten Moritz, Dr.-Ing. Lars Schiemann, 2010.

Load assumption for each condition can be summarized as in table 2.6

Table 2.6 Load combination and load cases

Membrane area $= 167.86 \text{ m}^2$

Material

For the selection of the materials, different criterions were taken into account:

- The operation of the pool deck must be disturbed as little as possible by maintenance and cleaning works.

- The durability of the material under high wind loads or rain and must be able to bear foot traffic which could occur by workers.

- The fire prevention authorities required a non-combustible material.

- The material must not to increase overall cost greater than the budget.

Due to the above the above specifications, a PVC-coated Polyester fabric was selected.

1. Steel Structure

Steel section were composed as rigid frames used in this project as a primary structure which are mainly subjected to compressive force as well as bending moment and the torsion forces. (figure 2.10)

The performance of steel works was analyzed according to the AISC, American Institute of Steel Construction and ASTM A36 Code.

2. Cable

Cables used as tension structure in this project are mentioned by AISI 304, and can be classified in three sizes, dia. 8 mm., 7 mm., and 6 mm. with breaking load of 86 kN, 35 kN, and 28 kN respectively. All of them are 1×19 spiral strand cable used as boundary cable and tie down cable.

The force analysis in each cable was studied by the computational model (Forten 4000) with various load cases and will be discussed later on. The state of Rigure 2.11 Series of Rigid frames as fixed to the site

Figure 2.10 Rigid frame as a primary structure

Figure 2.12 Steel cable with Thread terminal Figure 2.13 PVC coated polyester membrane

3. Membrane

According to the membrane stress analysis (see further in p.67), the fabric material which has been selected for this project is PVC coated polyester because it has a good tensile strength and elasticity. Furthermore, it is relatively inexpensive and has a flexibility and ease of handling that suitable for erection. The basic properties of PVC coated polyester fabric are

Table 2.7 Basic properties of PVC coated polyester fabric.

Werner Sobek e.a.: Von der Faser zum Gewebe. (1993).

3. EVALUATION OF STATIC ANALYSIS

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SERVICEABILITY LIMIT STATE (SLS)

To satisfy the Serviceability limit state criteria, a structure must remain functional for its intended use subjected to routine loading. A structure is deemed to satisfy the serviceability limit state when the constituent elements do not deflect by more than certain limit as mentioned in the building codes.

At the SLS it shall be verified that:

 $\mathsf{E}^{}_{\mathsf{d}} \,\leq\, \mathsf{C}^{}_{\mathsf{d}}$

where E_d is the design value of the effects of actions specified in the serviceability criterion, determined on the basis of the relevant combination.

> C_{d} is the limiting design value of the relevant serviceability criterion.

For serviceability limit state, the partial factor $(γ)$ should be taken as 1.0

Load cases and factor for serviceability limit state

A) Membrane Stress (S11)

membrane sigma 11 stresses (KN/m)

Figure 3.1 LC0: Membrane Stress 11

B) Membrane Stress (S22)

membrane sigma 22 stresses (KN/m)

Figure 3.2 LC0: Membrane Stress 22

LC 0 : FORM FINDING

C) Cable Axial Forces: *Tie down cables*

D) Cable Axial Forces: *Boundary cables*

A) Membrane Stress (S11)

membrane sigma 11 stresses (KN/m)

B) Membrane Stress (S22)

 $(min): -0.0000$

membrane sigma 22 stresses (KN/m)

C) Membrane Deformation

Node displacements (m)

<code>LC 1 - SLS</code> : $~$ 1.0 <code>DL + 1.0 V $_{\rm 0}$ </code>

D) Cable Axial Forces: *Tie down cables*

E) Cable Axial Forces: *Boundary cables*

A) Membrane Stress (S11)

membrane sigma 11 stresses (KN/m)

B) Membrane Stress (S22)

 $(min): -0.0000$

membrane sigma 22 stresses (KN/m)

C) Membrane Deformation

Node displacements (m)

D) Cable Axial Forces: *Tie down cables*

No.	Axial Force		No.	Axial Force
	(kN)			(kN)
$\pmb{0}$	1.03		19	1.20
$\mathbf{1}$	1.08		$20\,$	1.11
$\overline{\mathbf{c}}$	1.05		21	1.21
$\mathbf{3}$	1.10		22	1.13
$\overline{\mathbf{4}}$	1.04		23	1.21
5	1.09		24	1.11
6	1.03		25	1.22
$\boldsymbol{7}$	1.09			
8	1.04			
9	1.10			
$10\,$	1.03			
11	1.10			
12	1.05			
13	1.13			
14	4.81			
15	1.36			
16	4.90			
$17\,$	1.41			
18	1.13			

E) Cable Axial Forces: *Boundary cables*

<code>LC 3</code> - <code>SLS</code> : $~$ 1.0 <code>DL + 1.0 V $_{\rm 0}$ + 1.0 WL</code>

A) Membrane Stress (S11)

0.7180

membrane sigma 11 stresses (KN/m)

<code>LC 3</code> - <code>SLS</code> : $~$ 1.0 <code>DL + 1.0 V $_{\rm 0}$ + 1.0 WL</code>

B) Membrane Stress (S22)

 $(min): -0.0000$

membrane sigma 22 stresses (KN/m)

<code>LC 3</code> - <code>SLS</code> : $~$ 1.0 <code>DL + 1.0 V $_{\rm 0}$ + 1.0 WL</code>

C) Membrane Deformation

Node displacements (m)

<code>LC 3</code> - <code>SLS</code> : $~$ 1.0 <code>DL + 1.0 V $_{\rm 0}$ + 1.0 WL</code>

D) Cable Axial Forces: *Tie down cables*

E) Cable Axial Forces: *Boundary cables*

A) Membrane Stress (S11)

membrane sigma 11 stresses (KN/m)

B) Membrane Stress (S22)

 $(min): -0.0000$

membrane sigma 22 stresses (KN/m)

C) Membrane Deformation

Node displacements (m)

D) Cable Axial Forces: *Tie down cables*

E) Cable Axial Forces: *Boundary cables*

<code>LC 2 - SLS:1.0 DL + 1.0 V $_{\rm 0}$ + 1.0 LL</code>

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<code>LC 3</code> - <code>SLS</code> : $~$ 1.0 <code>DL + 1.0 V $_{\rm 0}$ + 1.0 WL</code>

LC 4 - SLS : $~$ 1.0 DL + 1.0 V_0 + 1.0 LL + 1.0 WLL

Ultimate Limit State (ULS)

To satisfy the Ultimate limit state, the structure must not collapse when subjected to the peak design load. A structure is deemed to satisfy the ultimate limit state criteria if all factored: bending, shear, and tensile or compressive stresses are below the factored resistance calculated for the member under consideration.

At the ULS it shall be verified that:

 $\mathsf{E}^{}_{\mathsf{d}} \,\leq\, \mathsf{R}^{}_{\mathsf{d}}$

where

 E_d is the design value of the effects of actions such as internal force, moment or a vector representing several internal forces or moments.

 R_{d} is the design value of the corresponding resistance.

For ultimate limit state, the partial factor (γ) for actions and combinations of actions are obtained from EN 1990 or CRO-2005

Permanent actions: $\;\;\gamma_{\rm G}\;\;$ = 1.35 Variable actions: γ_{o} = 1.50

Load cases and factor for ultimate limit state

A) Membrane Stress (S11)

0.6000

membrane sigma 11 stresses (KN/m)

B) Membrane Stress (S22)

 $(min): -0.0000$

membrane sigma 22 stresses (KN/m)

C) Membrane Deformation

Node displacements (m)

D) Cable Axial Forces: *Tie down cables*

E) Cable Axial Forces: *Boundary cables*

A) Membrane Stress (S11)

membrane sigma 11 stresses (KN/m)

B) Membrane Stress (S22)

membrane sigma 22 stresses (KN/m)

C) Membrane Deformation

Node displacements (m)

D) Cable Axial Forces: *Tie down cables*

E) Cable Axial Forces: *Boundary cables*

A) Membrane Stress (S11)

0.6682

membrane sigma 11 stresses (KN/m)

B) Membrane Stress (S22)

 $(min): -0.0000$

membrane sigma 22 stresses (KN/m)

C) Membrane Deformation

Node displacements (m)

D) Cable Axial Forces: *Tie down cables*

E) Cable Axial Forces: *Boundary cables*

A) Membrane Stress (S11)

membrane sigma 11 stresses (KN/m)

B) Membrane Stress (S22)

 $(min): -0.0000$

membrane sigma 22 stresses (KN/m)

C) Membrane Deformation

Node displacements (m)

D) Cable Axial Forces: *Tie down cables*

E) Cable Axial Forces: *Boundary cables*

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LOAD CASE	SLS			ULS		
	$S11*$	$S22*$	Node displacement	$S11*$	$S22*$	Node displacement
	$kN·m-1$	$kN·m-1$	cm.	$kN·m-1$	$kN·m-1$	cm.
LC ₁	5.61	0.87	1.67	5.51	0.92	2.24
LC ₂	5.95	1.15	16.01	7.66	1.28	19.20
LC ₃	6.29	2.09	19.10	36.90	5.93	23.90
LC ₄	6.85	1.87	13.50	18.70	4.32	15.90

Table 3.27 Membrane Stress and Deformed Shape Summary

** The maximum value selected from the average stress of majority area on membrane.*

Table 3.28 Maximum Tension Forces in Cables Summary

Structural Dimensioning: *MEMBRANE*

membrane sigma 11 stresses (KN/m)

Tensile stength (Weft) for PVC type $I \rightarrow 2800$ N/5cm = 56 kN/m > 36.90

For the area around the corner of membrane, pulled down by the cables, which has the stress high as 86 kN/m (figure A) will be reinforced by the corner plate and doubling membranes.

According to the Mechanical values of fabric (Appendix IV). Therefore, the PVC-coated polyester fabric type I with breaking tensile strength of 2500/2500 N/5cm was selected to use.

Selected Material

Structural Dimensioning: *Cables*

1. Boundary Cable

Tension force analysis in cables was studied in the numerical models. The boundary cables are classified in to two groups due to the membrane geometry. The first is BC 01 and BC 02 , which links the membrane between cantilever beams, and the another group is BC 03 – BC 19 which links the membrane between columns (Figure 3.1).

The maximum forces in boundary cable occur in Load case 03 (ULS) when wind load was assigned with the partial factor of 1.50. As presented in the table 3.29, the highest value for BC01, BC02 is 67.72 kN and for BC03 – BC19 is 27.80 kN. According to Appendix IV, Typical grade 316 stainless steel – Spiral strand 1×19 was selected, and use wire with diameter of 8 mm. and 6 mm. for BC01, BC02 and BC03 – BC19, respectively.

The details and specifications of boundary cable are presented in section 6.

Figure 3.2 Boundary cables and Thread toggle

2. Tie Cable

Table 3.30 Maximum Tension Forces Tie Cables (LC3 – ULS)

The maximum force in tie cable also occurs in Load case 03. As can be seen from the table 3.30, TC 12 has the greatest tension force of 34.60 kN. Thus, the stainless steel spiral strand 1 \times 19 diameter 7 mm. with breaking load of 35 kN (Appendix IV) was chosen.

STRUCTURAL DIMENSIONING: Corner Plates

Figure 3.3 Forces act on Corner plate

The forces are not transferred directly into the corner by the fabric, but are passed into to the boundary cables and then transfer those forces to the corner plates.

 F_{C1} : Tension force in Boundary cable (Left) \quad = 15 kN $\mathsf{F}_{\texttt{C2}}$: Tension force in Boundary cable (Right) = 15 kN F_{L} $\;$: Tension force in Link $\;$ = 17.5 kN Fy : Tensile yield strength = $2500 \text{ kg} \cdot \text{cm}^{-2}$ Ag : Gross section area (cm²) Corner plate thickness = 8 mm. L : Length of barrel pipe $= 4.2$ cm. Strength of fillet weld for E70, 6mm. = $(0.707)(0.6)(0.3)(4900) = 620$ kg·cm⁻¹

1. Check tensile strength of plate

Allowable Force (P_A) = 0.6 Fy Ag $= (0.6)(2500)(8 \times 0.8)$ $=$ 9600 kg = 96 kN PA (96 kN) > FL (17.5 kN) **OK**

2. Determine the weld size for barrel pipe to corner plate (point A)

- Shear stress at point A (f_s) = F_{C2} / ΣL
	- $=$ (1500) / (2 \times 4.2)
	- = 178.57 kg⋅cm-¹

Assume the welds are 6 mm. fillet with E70 electrodes.Hence, the leg size of weld is:

- $=$ (178.57) / (620)
- $=$ 0.28 cm \sim 3 mm. (both sides)

4. PATTERNING

PATTERNING CRITERIA

The membrane patterning is one of the important procedure in membrane designing. It is allows a three-dimensional shape of surface translate into twodimensional cutting pattern, in order to enable the manufacture of the membrane. The patterns are made out of strips of fabric of $1.0 - 1.8$ m. wide, as shown in the next pages, and keep the cutting out waste as low as possible. After cutting, the strips were assembled by welding and then were transported to the erection site.

Because of flexure and elasticity of material, the membrane has to be compensated (made smaller) so that when it is installed, it can be achieve predefined stress and correct geometry. To determine the compensation values, data from strain test under the applied tension and the reduction of stress over the lifetime were investigated.

According to the Biaxial test of PVC Precontraint 702 reported by Blum Laboraory, the resulting of stress and strain under different load levels were determined as shown in figuer 4.1 and 4.2 repectively. For the compensation, the strain values at the end of the test were adopted.

Ferrari Precontraint 702 – Type I

Warp : 2.0 % Weft : 1.8 %

Seam welding width* : for PVC 40 – 80 mm. (use 75 mm.) ** Data gained from the Lecture: OM 02 Studio Patterning and Detailing*

Figure 4.2 Load history with strain measurement

PATTERNING OVERVIEW

Patterning : Part B

Patterning : Part B

Patterning : Part C

EDGE CONNECTIONS

There are 2 different edges and welded seam which need to be considered as the following:

- (A), (B) Flexible edge: for boundary cable \varnothing 6mm, and \varnothing 8 mm.
- (C) Membrane seam : Overlapping welded
- (D), (E) Clamping plate edge: which membrane edge are fixed to the primary structure.

A – B. Flexible edge: for boundary cable

The flexible edge used in this project is the cable edge, where the boundary cable runs in the pocket edge of the membrane and collects the forces from the fabric and then leads them to the primary structure. There is a recommendation* that the limitation of the splay angle in the pocket should not be over than 15° in order to avoid the weld seam peeling off.

** Michael Seidel, Tensile Surface Structures, pp.76*

Figure 4.4 Membrane Edge Detailing

C. Welded seams

Membrane strips were joined by welding. Welded seams are made by overlapping of the edge membranes and then weld them together. In this case, the seam width is about 75 millimeter.

EDGE CONNECTIONS

D. Clamping Plate Edge I

At the edge (Figure 4.5), it is provided as a fixed edge anchor of the membrane to the primary structure. The fabric has a keder rail at the end and is clamped by the aluminium profile bar which is connected to the steel structure by bolts. (The drawing and detailing are shown in section 6.)

Figure 4.5 Clamping Plate Edge (I)

E. Clamping Plate Edge II

This type of edge was used at the central-part membrane (Part C), in order to stiff membrane edge. The fabric strip to be connected has a keder and was pressed to the primary structure by aluminium flat bars which were fixed along their length by bolts. (The drawing and detailing are also shown in section 6.)

Figure 4.6 Clamping Plate Edge (II)

5. COST ESTIMATION & ERECTION PROCESS

COST ESTIMATION

1. Material Specification

The materials used in this project are presented in the table 5.1.

Table 5.1 Material Specification

2. Scope of Works

The following items are included for price estimation.

- A. Design and calculating of membrane and support structure.
- B. Manufacturing and supplying for the following items:
	- 1) Membrane fabric
	- 2) Cable (Stainless steel 304)
	- 3) Tendon (Stainless steel, 304)
	- 4) Corner plate (Stainless steel) and aluminium clamping
	- 5) Gasket (Neoprene)
	- 6) Nuts & Bolts

The following items are excluded from price estimation.

- A. Steel structure and civil works.*
- B. Water supply and electricity at the site work.
- C. Cranes and lifting devices.
- D. VAT 7%
- E. Installation work.

(* The reaction force report are provided for client or engineer if they require).

3. Schedule

Because the pool was in operation during the entire erection phase, thus the schedule timeline of erection is very tight and precise. The total erection days used are approximate 65 days as shown in the table 5.2.

4. Cost Estimation

The estimated price for this project is approximate 20,000 EU or 118.34 EU/m² (only the include items were take into account as described before). The cost estimation is presented in particularly in the table 5.3.

COST ESTIMATION

Table 5.2 Erection timeline

COST ESTIMATION

Total project's covered area : 169 sq.m.

Total membrane area : 185 sq.m.

Rate exchange 1 Euro (EU) = 42.47 Baht Thai (BHT)

Table 5.3 Cost Estimation

ERECTION PROCESS

A. Steel structure installation

Figure 5.1 (A)-(B) The steel structure was erected to its position as primary structure.

B. Installation of central membrane (Part C)

The erection of the central membrane was the first carried out after the completion of primary structure. Membrane was folded out and brought up to its position (figure 5.2).

Figure 5.2

 (A) (B)

Figure 5.3 (A)-(B)

In order to avoid damage to the fabric, the strips of neoprene were inserted underneath (between the membrane and the beam) and upper (between membrane and aluminium bar) before fixing edge.

 (A) (B) (C)

- (A) Installation of aluminium flat bar as a clamping plate edge.
- (B) A worker fixed the membrane edge to the supported beam by bolts.
- (C) Clamping plate edge after finished.

 (A) (B) (C)

Figure 5.5

(A) Membrane was temporary stabilized by tie to the column.

- (B) Workers fixed the corner area with corner plate and connected it to the column as a high point.
- (C) Corner detail after completely finished.

Figure 5.6

(A) Corner plate was installed to the membrane.

(B) 7 mm. Tie cable was threaded through a toggle and both of its end were tied down and connected to the bases of columns.

Figure 5.7

(A) Tensioning the cable by Ratchet lever hoist.

(B) Central membrane after tensioning.

C. Installation of wings membrane (Part A and B)

Sheets of plastic were laid out on the steel beams before put the fabric on, which prevent the membrane from damage during the erecting operation (figure 5.8). These sheets will be removed when the erection is accomplished.

Figure 5.8

Figure 5.9

(A) Membrane fabric was folded out and prepared for erecting.

(B) After the fabric has been unfolded, then it was lifted up to its position, and then was expanded to cover the intended area.

 (A) (B)

Figure 5.10

- (A) Aluminium clamping profile and membrane edge were assembled together before connecting to the beam
- $(B) (C)$ After lifting the clamping profile to the intended position, then it was connected to the beam by bolting along the length while the boundary cable was also introduced through the pocket.

Figure 5.11

- (A) Another edge of membrane is reached to the beam and joined as a stiff edge.
- (B) The boundary cable was connected to the column by holding of the thread toggle.

(A) The corner plate was assembled and temporary stabilized by pulling rope.

(B) A worker assemble the tie-down cable to the corner plate.

(C) Tie-down cable is connected at the column and tensioned by lever hoist.

(C)

Figure 5.13

- (A) To apply force to the membrane, the boundary cable was also tensioned by lever hoist.
- (B) A worker was disappearing the wrinkles on the membrane surface.

 (A) (B)

Figure 5.14

The same procedure of erecting was repeated for the remain part of the roof.

Membrane roof after finishing (A) On the north wing

(B) On the west wing

 (A) (A)

6. DRAWINGS & DETAILING

LAY OUT

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PLAN

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SECTION A

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Detail B1 Steel structure $1200.30 = 3.60$ Hole \oslash 30 mm.
for boundary cable \oslash 6 mm. Hole \oslash 30 mm. for boundary cable \oslash 10 mm. Fix by Bolts M10 @ 300 mm. Aluminium extrude **ELEVATION** $Scale$ $1:20$ \vartriangle ⚠ Swaps Thread
for boundary cable (2010 mm $\begin{array}{c} \textbf{Supp} \textbf{ The} \textbf{a} \\ \textbf{for boundary} \textbf{ can be } \oslash \textbf{a} \textbf{ are} \end{array}$ 0.10 Patric roof Surbal ⊘38 n .
Surball ⊘38 mm - Boine 1410 @ 300 mm Abuntabum extra

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$$
\begin{array}{cc}\n\text{TOP} & \text{VIEW} \\
\hline\n\text{Scale} & 1:5\n\end{array}
$$

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Corner Plate (CP1)

Corner Plate (CP2)

Corner Plate (CP3)

Corner Plate (CP3) $Scale$ $\overline{1:5}$

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1. Boundary Cable ∅ *8 mm.*

2. Boundary Cable \varnothing 6 mm.

3. Tie Down Cable ∅ *7 mm.*

SWAGE JAW

4. Thread Toggle

Link Corner Scale $\overline{1:5}$

1. No Column required as drawn marked C101

SECTION B $1:5$ Scale

1. No Column required as drawn marked C102

Scale

 $1.5\,$

SECTION B Scale $1:5$

1. No Column required as drawn marked C104

1. No Column required as drawn marked C106

SECTION A Scale $1:5$

10. No Column required as drawn marked C107

SECTION A Scale $1:5$

1. No Beam required as drawn marked B209

Scale $1:15$

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1. No Beam required as drawn marked B212

Scale 1.15

SECTION A Scale $1:5$

9. No Beam required as drawn marked B215

Scale $1:10$

Scale

 $1:10$

2. No Beam required as drawn marked B216

13. No Plate required as drawn marked PL101

Scale 1 : 5

Plate Detail

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Plate Detail

Plate Detail: $P105$
Thk. = 8.0 mm. Scale $1:25$

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7. APPENDIXES

APPENDIX I

Minimum Design Live Load in Thailand

Table 1. Minimum Live Load for Building Design

APPENDIX II

Wind loading Code for Building Design in Thailand

There are three different procedures for determining wind loads on building as follows:

1. *Simple procedure*

This procedure is appropriate for use with the majority of wind loading applications, including the structure and cladding of low and medium rise building and the cladding design of high rise building. This procedure suit for the building which its height should not greater than 80 m. nor more than 4 times of the their minimum effective width ($h < 80$ m. or $h < 4w$).

2. *Detailed procedure*

This procedure is appropriate for building whose height is greater than 4 times of their minimum effective width or greater than 80 m (h $>$ 80 m. or h $>$ 4w) and other buildings whose light weight, low frequency and low damping properties make them susceptible to vibration.

3. *Wind tunnel test procedure*

This procedure is appropriate when more exact definition of dynamic response is needed and for determining exterior pressure coefficients for cladding design on buildings whose geometry deviates markedly from more common shapes for which information is already available.

At present, the wind loading standard for building design in Thailand follows the DPT standard 1311-50 which is revised and published by Department of Public Works and Town & Country Planning. The reference wind speed is based on the study of the wind climate in Thailand.

Specified Wind Loading

where

The specified external pressure or suction due to wind on surface of a building can be determined from:

 $p = (I_w)(q)(Ce)(Cg)(Cp)$ (1)

0.75

0.75

: Essential facilities, including hospitals, fire and police stations, national defense facilities and shelters, communication centers, power stations, and utilities required in an emergency. Post-diaster (Category IV)

Reference Velocity Pressure

The reference wind velocity can be determined from the following equation:

$$
q~=~\frac{1}{2}\bigg(\frac{\rho}{g}\bigg)\overline{V}^2
$$

- Where $q =$ Reference wind velocity pressure $(kg·m⁻²)$
	- ρ = Air density (1.25 kg⋅m⁻³)
	- $g =$ Acceleration due to gravity (9.81 m⋅s⁻²)
	- $V =$ Design wind speed $(m·s⁻²)$
		- \overline{V} $= V_{50}$ for serviceability limit state
		- $\overline{\mathsf{v}}$ $=$ $T_F \cdot V_{50}$ for ultimate (strength) limit state
		- V_{50} = Reference wind speed that is based on one-hour average wind speed at 10 m. in open terrain in 50-years return period. V_{50} and Typhoon Factor (T_F) are shown in Table 2. and Figure 2.

(2)

Table 2. Reference Wind Speed and Typhoon Factor

Figure 2. Reference Wind speed zone for Thailand

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Exposure Factor

The exposure factor (Ce) reflects changes in wind speed and height, and also the effects of variations in the surrounding terrain and topography.

Simple Procedure

The exposure factor can be determined from equation 3 or in Table 3.

$$
Ce = \left(\frac{Z}{10}\right)^{0.2}, \quad Ce \ge 0.9
$$
 (3)

where Z is the reference height above ground (m).

Detailed Procedure

The exposure factor is based on the mean wind speed profile, which varies considerable depending on the general roughness of the terrain over which the wind has been blowing before it reaches the building. To determine the exposure factor, three categories have been established as follows:

Exposure A: (Open or standard exposure) open level terrain with only scattered buildings, trees or obstructions, open water or shorelines thereof.

$$
Ce = \left(\frac{Z}{10}\right)^{0.28}, \quad Ce \ge 1.0 \tag{4}
$$

Exposure B: Suburban and urban areas, wooded terrain or centers of large towns.

$$
\text{Ce} = 0.5 \left(\frac{Z}{10} \right)^{0.5}, \quad \text{Ce} \ge 0.5 \tag{5}
$$

Exposure C: Centers of large cities with heavy concentrations of tall buildings. At least 50% of the buildings should exceed 4 storeys.

$$
\text{Ce} = 0.4 \left(\frac{Z}{10} \right)^{0.72}, \quad \text{Ce} \ge 0.4 \tag{6}
$$

where Z is the reference height above ground (m).

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Table 3. Exposure Factors (Ce)

Gust Effect Factor

The gust effect factor (Cg) is the ratio of the maximum effect of the loading to the mean effect of the loading.

Simple Procedure

The gust effect factor is one of the following values:

- a) 1.0 or 2.0 for internal pressures as appropriate¹.
- b) 2.0 for the building as a whole and main structural members.
- c) 2.5 for small elements including cladding.

Detailed Procedure

The gust effect factor can be expressed as:

$$
Cg = 1 + g_p \left(\frac{\sigma}{\mu}\right) \tag{7}
$$

- where Cq = Gust Effect Factor
	- g_p = A statistical peak factor for the loading effect obtained from figure in the code.
	- μ = The mean loading effect.
	- σ= The "root-mean square" loading effect.

Figure 3. Wind flow around the building *Source: http://www.cleanfieldenergy.com/how_VAWTs_work.php*

1 Engineering Institute of Thailand (2003), E.I.T. Standard 1018-46, Wind Loading Code for Building Design.

Pressure Coefficients

Pressure coefficients are the non-dimensional ratios of wind-induced pressures on a building to the dynamic pressure of the wind speed at the reference height. Pressures on the surfaces of structures vary considerably with the shape, wind direction and profile of the wind velocity.

The values of Cp for windward and leeward roof are given in the following Table:

Figure 4. Cp for External Pressure of building

Table 4. Windward and Leeward Roof Pressure Coefficients, Cp, θ > 10°

Table 4.(Continue) Windward and Leeward Roof Pressure Coefficients, Cp, θ < 10°

APPENDIX III

Mechanical Values of Membrane

APPENDIX IV

Wire Breaking Load Ratings & Comparison Table

TYPICAL GRADE 316 STAINLESS STEEL WIRE ROPE BREAKING LOADS

THREAD TERMINALS

Stainless Steel - AISI 316

Note: All breakloads are determined by thread

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RIGGING SCREWS TOGGLE-TERMINAL

High Polished Stainless Steel - AISI 316

Note: All breakloads are determined by Clevis Pin (D1) & thread

Bodie with bronze inserts

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APPENDIX V: *REACTION FORCES IN COLUMNS*

Case 0: LC 0 (Form finding)

SLS - LC 1: 1.0DL + $1.0V_0$

 $\sqrt{3}$ $\sqrt{1}$ $\sqrt{1}$ $\sqrt{1}$

SLS - LC 2: $1.0DL + 1.0V_0 + 1.0LL$

SLS - LC 3: 1.0DL + $1.0V_0 + 1.0WL$

SLS - LC 4: 1.0DL + $1.0V_0 + 1.0WL + 1.0LL$

ULS - LC 1: 1.35DL + $1.35V_0$

ULS - LC 2: 1.35DL + $1.35V_0$ + 1.50LL

ULS - LC 3: 1.35DL + $1.35V_0 + 1.50WL$

ULS - LC 4: 1.35DL + $1.35V_0 + 1.35LL + 1.35WL$

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